

LOCATION MAP

PROJECT DESCRIPTION NOTE:

THE FOLLOWING PROJECT GENERALLY CONSISTS OF THE INSTALLATION OF A RETAINING WALL NEAR 754 NORTH POND COURT AND @ 768 SOUTH PONT COURT, LAFAYETTE, CA.

PROJECT OWNER AND ADDRESS

APN: 365-530-022 - 754 NORTH POND COURT APN: 365-530-016 - 768 SOUTH POND COURT

CALIFORNIA RESERVE HOA

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LEGEND:			
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EXISTING	(Symbol Size May Vary)	PROPOSED
	PROPERTY BOUNDARY	
	CURB & GUTTER	
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	LIMIT OF GRADING	
	DIRECTION OF SURFACE DRAINAGE	•

RECEIVED on 08/05/2024 CDDP24-03004 By Contra Costa County Department of Conservation and Development RETAINING WALL CALIFORNIA RESERVE HOA 754 N POND COURT & 768 S POND CT

-AREA OF PROPOSED WORK LAFAYETTE CA 94549 AREA OF PROPOSED WORK

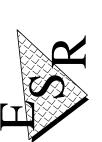
SITE PLAN

ABBREVIATIONS:

CO DS FL HP LP PL	CLEANOUT DOWNSPOUT FLOW LINE HIGH POINT LOW POINT PROPERTY LINE	PVC PUE RCP SD SDMH TC	POLY VINYL CHLORIDE PUBLIC UTILITY EASEMENT REINFORCED CONCRETE PIPE STORM DRAIN STORM DRAIN MANHOLE TOP OF CURB

GEOTECHNICAL ENGINEER OF RECORD GEOFORENSICS, INC. 303 VINTAGE PARK

DRIVE STE. 220 FOSTER CITY CA.





PROJECT# 3684-05

SHEET

S-1 1 OF 15 SHEETS

ANY ENGINEERING DESIGN NOTES AND SPECIFICATIONS PRESENTED IN THIS PLAN SET ARE CONTINGENT UPON OUR FIRM BEING CONSULTED WHEN ANY QUESTIONS ARISE WITH REGARD TO THE NOTES AND SPECIFICATIONS CONTAINED HEREIN, AND TO PROVIDE TESTING AND INSPECTION SERVICES FOR CONSTRUCTION OPERATIONS. UNANTICIPATED SOIL AND GEOLOGIC CONDITIONS ARE COMMONLY ENCOUNTERED DURING CONSTRUCTION. WHICH CANNOT BE FULLY DETERMINED FROM EXISTING EXPOSURES OR BY LIMITED SUBSURFACE INVESTIGATION. SUCH CONDITIONS MAY REQUIRE ADDITIONAL EXPENDITURES DURING CONSTRUCTION TO OBTAIN A PROPERLY CONSTRUCTED PROJECT.

GENERAL NOTES

- IT SHALL BE UNDERSTOOD THAT THE TERM OWNER AS USED HEREIN IS THE CALIFORNIA RESERVE HOA @ 745 N POND CT AND 768 S POND CT LAFAYETTE, CA 94549.
- 2. IT SHALL BE UNDERSTOOD THAT THE TERM COUNTY AS USED HEREIN IS THE CONTRA COSTA COUNTY OR ITS AUTHORIZED REPRESENTATIVE.
- 3. IT SHALL BE UNDERSTOOD THE TERM ENGINEER IS THE DESIGN CIVIL ENGINEER, CHRISTOPHER L. WILHITE, OR HIS AUTHORIZED REPRESENTATIVE.
- 4. IT SHALL BE UNDERSTOOD THAT THE TERM ESR AS USED HEREIN IS THE ENGINEER OF RECORD, ENGINEERED SOIL REPAIRS, INC
- ANY DEVIATION FROM THE APPROVED PLANS DURING CONSTRUCTION WILL REQUIRE 24 HOURS PRIOR NOTICE TO THE ENGINEER. AT LEAST ONE SET OF PLANS SHALL BE ON SITE STRUCTURAL STEEL NOTES: AT ALL TIMES FOR INSPECTION.
- NO WORK WHATSOEVER SHALL BE COMMENCED WITHOUT FIRST NOTIFYING THE COUNTY, THE OWNER AND THE ENGINEER.
- 7. IT SHALL BE ESR'S RESPONSIBILITY TO COORDINATE INSPECTIONS WITH THE COUNTY AND THE ENGINEER.
- PROTECTIVE FENCING AND/OR BARRIERS SHALL BE PROVIDED WHEN NECESSARY TO PROTECT ADJACENT PROPERTIES DURING THE GRADING OPERATION
- ALL MATERIALS, METHODS AND WORK TO BE IN ACCORDANCE WITH THE PROJECT DRAWINGS AND SPECIFICATIONS AS WELL AS THE STANDARD PROVISIONS OF THE STANDARD PROVISIONS OF THE CONTRA COSTA COUNTY, THE 2022 CBC WITH ALL APPLICABLE AMENDMENTS AND UPDATES.
- A PERMIT, AN APPROVED BACKFLOW PREVENTION DEVICE AND A METER ARE REQUIRED FOR TEMPORARY CONSTRUCTION WATER FROM FIRE HYDRANT AND OR EXISTING WATER SERVICE DURING CONSTRUCTION.
- 11. ESR SHALL PROVIDE THE OWNER AND ENGINEER WITH THE NAME AND TELEPHONE $_{
 m 3.}$ NUMBERS OF THE RESPONSIBLE PERSON TO CONTACT, WITH REGARD TO THIS PROJECT, 24 HOURS A DAY.
- CONSTRUCTION WORK SHALL OCCUR ONLY BETWEEN THE HOURS OF 7:30 A.M TO 5:00 P.M. MONDAY THROUGH FRIDAY (NOT INCLUDING HOLIDAYS), UNLESS AN EXCEPTION IS GRANTED BY THE COUNTY. EXCEPTIONS WILL BE CONSIDERED ONLY, IN THE OPINION OF THE COUNTY, IF CONSTRUCTION DURING THE ABOVE PERIOD WOULD INCONVENIENCE THE PUBLIC AND NEIGHBORING RESIDENTS MORE THAN WORKING AT OTHER HOURS OR ON WEEKENDS.
- 13. ESR SHALL PROVIDE ADEQUATE DUST CONTROL AT ALL TIMES AS REQUIRED BY THE OWNER'S REPRESENTATIVE AND THE CONTRA COSTA COUNTY. ANY OPERATION THAT CREATES EXCESSIVE DUST SHALL CEASE IMMEDIATELY UNTIL SUFFICIENT MEASURES, SATISFACTORY TO THE OWNER'S REPRESENTATIVE, AND THE COUNTY HAVE BEEN TAKEN TO INSURE COMPLIANCE WITH DUST CONTROL REQUIREMENTS.
- 14. ESR SHALL FURNISH AND INSTALL ALL SIGNS, LIGHTS, BARRICADES, AND OTHER TRAFFIC CONTROL OR WARNING DEVICES, INCLUDING FLAG PERSON, AS REQUIRED BY THE COUNTY.

 3. PUMPED CONCRETE MAY BE USED AT THE CONTRACTOR'S OPTION. THE MAXIMUM
- 15. ALL WORK IS SUBJECT TO INSPECTION AND APPROVAL BY THE ENGINEER.
- CONTRACTOR IN THE FIELD.
- 17. EROSION CONTROL MEASURES SHALL BE EMPLOYED DURING ANY RAINY SEASON AS REQUIRED BY THE ENGINEERS AND/OR THE COUNTY.
- SEE CONTRACT DRAWINGS AND SPECIFICATIONS FOR ALL INFORMATION RELATIVE TO THE NEW AND EXISTING CONSTRUCTION AND CONDITIONS. RESOLVE CONFLICTS ON DRAWINGS WITH THE ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION.
 - a. ESR SHALL VERIFY THE LOCATION OF ALL UTILITIES AND SHALL PROTECT THEM FROM HARM AS REQUIRED TO PREVENT DAMAGE AND TO MAINTAIN THEIR USE.
- 20. ESR SHALL BE RESPONSIBLE FOR SITE CLEANUP TO THE SATISFACTION OF THE OWNER.

SPECIFICATIONS

STEEL REINFORCEMENT NOTES:

- STEEL REINFORCEMENT SHALL CONFORM TO ASTM A615, LATEST REVISION. USE GRADE 60 FOR #5 BARS AND GREATER. GRADE 40 CAN BE USED FOR #4 BARS AND LESS.
- 2. SHOP DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR APPROVAL AT LEAST 14 DAYS PRIOR TO INSTALLATION.
- MINIMUM CONCRETE COVER FOR REINFORCEMENT:
- a. FOOTINGS AND PIERS:
 - 3 INCHES AT THE BOTTOM AND SIDES
- b. #5 BAR AND SMALLER AT OTHER FORMED MEMBERS
 - 1 ½ INCHES TO FORM
- c. #6 BAR AND LARGER AT OTHER FORMED MEMBERS
 - 2 INCHES TO FORM
- 4. REINFORCEMENT SHALL BE PLACED TO CONFORM WITH "MANUAL OF STANDARD PRACTICES FOR DETAILING REINFORCED CONCRETE STRUCTURES" (ACI 315 AND 318). HOLD REINFORCEMENT IN ITS TRUE VERTICAL AND HORIZONTAL POSITION WITH DEVICES SUFFICIENTLY NUMEROUS TO PERMIT PLACEMENT OF CONCRETE WITHOUT DISPLACING THE REINFORCING STEEL. ALL HOOKS FOR STIRRUPS ETC., SHALL BE 135 DEGREE HOOKS.
- 5. THE CLEAR DISTANCE BETWEEN PARALLEL REINFORCEMENT IN LAYERS SHALL NOT BE LESS THAN 1-1/2 TIMES THE NOMINAL DIAMETER OF THE REINFORCEMENT, OR 1-1/3 TIMES THE MAXIMUM SIZE AGGREGATE, NO LESS THAN 1-1/2 INCHES.
- ALL REINFORCEMENT SHALL BE CONTINUOUS, STAGGER SPLICES WERE POSSIBLE. MINIMUM BAR LAP SHALL BE 40 BAR DIAMETERS, OR 24 INCHES (WHICHEVER IS GREATER) UNLESS 6. OTHERWISE SHOWN.

- 1. STRUCTURAL STEEL BEAMS SHALL BE 50-KSI STEEL
- 2. FABRICATION AND ERECTION SHALL COMPLY WITH AISC SPECIFICATIONS FOR DESIGN AND 8. FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST EDITION.
- WELDING SHALL CONFORM TO THE LATEST REVISIONS OF AWS D1.1 AND BE PERFORMED BY A CERTIFIED WELDER.
- SHOP DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR APPROVAL AT LEAST 14 DAYS PRIOR TO INSTALLATION

CONCRETE

- CONCRETE STRENGTH SHALL BE A MINIMUM OF 2,500 PSI.
- 2. PUMPED CONCRETE MAY BE USED AT THE CONTRACTOR'S OPTION. THE MAXIMUM AGGREGATE SIZE SHALL BE $\frac{3}{4}$ " AND SLUMP RANGE SHALL BE 4 TO 6 INCHES.
- CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. USE OF HOPPER, CHUTES OR TRUNKS OF VARYING LENGTHS SO THAT UNCONFINED FALL OF CONCRETE DOES NOT EXCEED 4 FEET. MECHANICAL VIBRATION FOR PROPER CONSOLIDATION IS REQUIRED.

DRILLED CONCRETE PIERS NOTES:

- CONCRETE PIERS SHALL BE INSTALLED AT THE LOCATIONS DETERMINED IN THE FIELD BY THE ENGINEER, APPROXIMATE LOCATIONS ARE SHOWN IN PLANS. CONCRETE PIERS SHALL HAVE A MINIMUM DIAMETER OF 24 INCHES AND A MINIMUM EMBEDMENT PER PLANS.
- IF REQUIRED, HOLES SHALL BE CASED TO PREVENT CAVING DURING DRILLING. CASING SHALL BE RETRACTED AS CONCRETE IS PLACED.
- PIERS SHALL BE DRILLED STRAIGHT AND PLUM (WITHIN 1% OF VERTICAL) AND SHOULD BE CLEANED OF LOOSE SOIL AND ROCK FRAGMENTS.
- 2. CONCRETE PLACEMENT SHOULD START AS SOON AS POSSIBLE AFTER DRILLING AND CLEANOUT IS COMPLETE. CONCRETE STRENGTH SHALL BE A MINIMUM OF 2.500 PSI.
- AGGREGATE SIZE SHALL BE $\frac{3}{4}$ " AND SLUMP RANGE SHALL BE 4 TO 6 INCHES.
- 16. LOCATIONS AND ELEVATIONS ARE APPROXIMATE AND SHALL BE VERIFIED BY THE 4. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. USE OF HOPPER, CHUTES OR TRUNKS OF VARYING LENGTHS SO THAT UNCONFINED FALL OF CONCRETE DOES NOT EXCEED 4 FEET. MECHANICAL VIBRATION FOR PROPER CONSOLIDATION IS REQUIRED.
 - 5. IF WATER IS PRESENT IN THE HOLE, TREMIE PIPE SHALL BE MAINTAINED AT LEAST 5 FEET BELOW THE SURFACE OF THE CONCRETE DURING CASTING OF THE PIER.
 - 6. AS CONCRETE IS PLACED. ANY CASING USED TO STABILIZE THE HOLE SHOULD BE WITHDRAWN. THE BOTTOM OF THE CASING SHOULD BE MAINTAINED NOT MORE THAN 5 FEET OR LESS THAN ONE FOOT BELOW THE LEVEL OF THE CONCRETE.

TIEBACK INSTALLATION NOTES:

- TIEBACKS SHALL BE INSTALLED AT THE LOCATIONS DETERMINED IN THE FIELD BY THE ENGINEER, APPROXIMATE LOCATIONS ARE SHOW IN PLANS. TIEBACKS SHALL BE DRILLED TO THE DEPTH REQUIRED.
 - a. IF REQUIRED, HOLES SHALL BE CASED TO PREVENT CAVING DURING DRILLING. CASING SHALL BE RETRACTED AS GROUT OR BACKFILL IS PLACED.
 - b. TIEBACKS SHALL BE FREE OF ALL LOOSE MATERIAL
- INSTALL DCP STEEL ROD OR STRANDS WITH CENTRALIZERS SPACED AT MAXIMUM 5-FOOT O.C. AND IMMEDIATELY FILL BORE HOLE WITH HIGH STRENGTH GROUT. TIEBACK BOLTS SHALL BE FULLY GROUTED FROM TOP TO BOTTOM OF HOLE. BONDED ZONE IS INDICATED ON DETAIL (PAGES 8-10).
- 3. TEST TIEBACKS AGAINST CONCRETE WALER ASSEMBLY FOR UPPER WALL AND AGAINST DRILLED PIER FOR THE LOWER STITCH PIERS.
- TESTING SHALL PROCEED AFTER GROUT IN THE PENETRATION LENGTH HAS ATTAINED THE APPROPRIATE COMPRESSIVE STRENGTH AS DETERMINED BY THE CONTRACTOR.
 - c. TIEBACKS SHALL BE STRESSED STRAIGHT AND TRUE AGAINST WALER. KINKING
- OR SHARP CURVATURE UNDER TENSION SHALL BE CAUSE FOR REJECTION. d. STRAND/ROD AND ROCK BOLT ASSEMBLY SHALL SHOW NO EXCESSIVE 1 MAX.) MOVEMENT AT TEST LOAD.
- HYDRAULIC JACKS SHALL BE CALIBRATED AND CERTIFIED WITHIN THE LAST SIX MONTHS BY AN INDEPENDENT TESTING AGENCY. CERTIFICATION SHALL BE PROVIDED TO THE ENGINEER PRIOR TO TESTING.
- 7. THE CONTRACTOR SHALL STOP EXCAVATION IF ADVERSE EFFECTS ON ADJACENT PROPERTIES ARE OBSERVED AND SHALL IMMEDIATELY NOTIFY THE COUNTY AND THE ENGINEER.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEETING THE PLAN DESIGN LOADS WITH RESPECT TO TIEBACKS.
- THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND APPROVAL ALL INFORMATION REGARDING THE TIEBACK INSTALLATION SYSTEM OTHER THAN AS SHOWN OR SPECIFIED ON THE DRAWINGS AND IN SPECIFICATIONS.
- 10. THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND APPROVAL ALL INFORMATION REGARDING TIEBACK ANCHORING DETAILS OTHER THAN SHOWN OR SPECIFIED ON THE DRAWINGS AND IN THE SPECIFICATIONS.
- 11. THE CONTRACTOR MAY INCREASE OR DECREASE THE ANGLE OF THE TIEBACKS BY APPROXIMATELY 5 DEGREES IF DEEMED NECESSARY BY THE FIELD SUPERINTENDENT TO MEET CONDITIONS IN THE FIELD WITH THE PRIOR APPROVAL OF THE ENGINEER.
- 12. THE CONTRACTOR SHALL INSTALL WEDGE WASHERS AS REQUIRED TO CORRECT MISALIGNMENT OF THE ANCHOR RODS WITH THE ANGEL SEATS.
- 13. TENSIONING OF TIEBACK: ALL TIEBACKS SHALL BE TENSIONED USING A CENTER HOLE HYDRAULIC JACK. WHEN THE GROUT HAS ATTAINED THE REQUIRED COMPRESSIVE STRENGTH, THE ANCHORS SHOULD BE PROOF TESTED TO 1.25 TIMES THE DESIGN LOAD AS OUTLINED IN THE LATEST EDITION OF THE POST-TENSIONING INSTITUTE MANUAL. PROOF TEST LOADS SHOULD BE HELD FOR 10 MINUTES, AND THE DEFLECTION AT TEST LOAD BETWEEN THE 1 AND 10 MINUTE READINGS SHOULD NOT EXCEED 0.04 INCHES. AFTER TESTING, THE TENSION IN THE ANCHOR SHOULD BE REDUCED TO THE DESIGN LOAD AND LOCKED OFF.
- 14. THE CONTRACTOR SHALL PROVIDE THE JACKS AND JACKING MECHANISMS USED IN THE STRESSING AND TESTING OF THE INSTALLED TIEBACKS. THE CONTRACTOR SHALL SUBMIT A RECENT (MAXIMUM 6 MONTHS OLD) CERTIFIED LOAD-PRESSURE TABLE FOR THE JACKS FOR REVIEW BY THE ENGINEER PRIOR TO TESTING.
- 15. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR RE-DRILLS OR REPLACEMENT OF TIEBACKS, WHICH FAIL TO MEET TEST LOADS, AND ALL THE COSTS INVOLVED.
- 16. CEMENT GROUT SHALL HAVE A MINIMUM OF 4,000 PSI.

TIE BACK LOADING:

TEST LOAD: ROW 1:62.7K

ROW 2: 137k LOCKOFF LOAD: ROW 1:50K

ROW 2: 109K

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Date: 05/16/2024

PROJECT # 3684-05

SHEET

S-2

GRADING SPECIFICATIONS NOTES

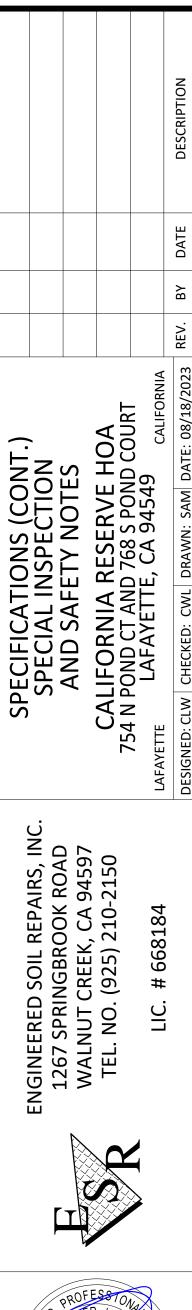
- 1. ALL GRADING, SITE PREPARATION, AND PLACING AND COMPACTION OF FILL SHALL BE DONE UNDER THE OBSERVATION OF THE ENGINEER OR HIS REPRESENTATIVE.
- 2. ALL FILL SHALL BE PLACED IN UNIFORM LIFTS NO GREATER THAN 8 INCHES IN UNCOMPACTED THICKNESS, MOISTURE CONDITIONED TO 2- TO 3- PERCENT OVER OPTIMUM MOISTURE CONTENT AND COMPACTED TO A MINIMUM DEGREE OF RELATIVE COMPACTION OF 90 PERCENT.
- 3. THE STANDARD TEST USED TO DEFINE MAXIMUM DENSITIES OF ALL COMPACTION WORK SHALL BE ASTM TEST PROCEDURE D1557. ALL DENSITIES SHALL BE EXPRESSED AS A RELATIVE COMPACTION IN TERMS OF THE MAXIMUM DRY DENSITY OBTAINED IN THE LABORATORY BY THE FOREGOING PROCEDURE.
- 4. COMPACTION SHALL BE BY FOOTED ROLLERS OR OTHER TYPES OF ACCEPTABLE COMPACTING ROLLERS. ROLLERS SHALL BE OF SUCH DESIGN THAT THEY WILL BE ABLE TO COMPACT THE FILL TO THE SPECIFIED DENSITY. ROLLING SHALL BE ACCOMPLISHED WHILE THE FILL MATERIAL IS WITHIN THE SPECIFIED MOISTURE CONTENT RANGE. ROLLING OF EACH LAYER SHALL BE CONTINUOUS OVER ITS ENTIRE AREA AND THE ROLLER SHALL MAKE SUFFICIENT TRIPS TO ENSURE THAT REQUIRED DENSITY HAS BEEN OBTAINED. NO PONDING OR JETTING SHALL BE PERMITTED.
- 5. COMPACTION SHALL BE BY FOOTED ROLLERS OR OTHER TYPES OF ACCEPTABLE COMPACTING ROLLERS. ROLLERS SHALL BE OF SUCH DESIGN THAT THEY WILL BE ABLE TO COMPACT THE FILL TO THE SPECIFIED DENSITY. ROLLING SHALL BE ACCOMPLISHED WHILE THE FILL MATERIAL IS WITHIN THE SPECIFIED MOISTURE CONTENT RANGE. ROLLING OF EACH LAYER SHALL BE CONTINUOUS OVER ITS ENTIRE AREA AND THE ROLLER SHALL MAKE SUFFICIENT TRIPS TO ENSURE THAT REQUIRED DENSITY HAS BEEN OBTAINED. NO PONDING OR JETTING SHALL BE PERMITTED.
- 6. WHEN FOOTED ROLLERS ARE USED FOR COMPACTION, THE DENSITY TESTS SHALL BE TAKEN IN THE COMPACTED MATERIAL BELOW THE SURFACE DISTRIBUTED BY THE ROLLER. WHEN THESE TESTS INDICATE THAT THE COMPACTION REQUIREMENTS ON ANY LAYER OF FILL OR PORTION THEREOF HAS NOT BEEN MET. THE PARTICULAR LAYER, OR PORTION THEREOF, SHALL BE REWORKED UNTIL THE COMPACTION REQUIREMENTS HAVE BEEN MET.
- 7. ALL FILL MATERIAL SHALL BE APPROVED BY THE SOIL ENGINEER.
- 8. ALL FILL MATERIAL SHALL BE A SOIL OR SOIL-ROCK MIXTURE THAT IS FREE FROM ORGANIC MATTER OR OTHER DELETERIOUS
- 9. FILL MATTER SHALL NOT CONTAIN ROCKS OR LUMPS OVER 6 INCHES IN GREATEST DIMENSION AND NOT MORE THAN 15% LARGER THAN 2-1/2 INCHES.
- 10. NO SOIL SHALL BE PLACED OR COMPACTED DURING PERIODS OF RAIN NOR ON GROUND THAT CONTAINS FREE WATER. SOIL WHICH HAS BEEN SOAKED AND WETTED BY ANY CAUSE SHALL NOT BE COMPACTED UNTIL COMPLETELY DRAINED AND UNTIL THE MOISTURE CONTENT IS WITHIN THE LIMITS APPROVED BY THE ENGINEER. APPROVAL BY THE ENGINEER SHALL NOT BE OBTAINED PRIOR TO THE GRADING OPERATION.
- 11. ALL CUT SLOPES EXPOSED DURING THE GRADING OPERATION SHALL BE INVESTIGATED BOTH DURING AND AFTER GRADING BY THE ENGINEER TO DETERMINE IF ANY SLOPE STABILITY PROBLEMS EXIST. SHOULD EXCAVATION DISCLOSE ANY GEOLOGICAL HAZARDS, THE ENGINEER SHALL RECOMMEND TREATMENT TO THE OWNER AND THE COUNTY FOR APPROVAL.
- 12. ALL GRADING AND SUBDRAIN INSTALLATION SHALL BE INSPECTED BY THE SOIL ENGINEER
- 13. WHEREVER SLOPE RATIOS ARE CALLED OUT, IT IS TO BE UNDERSTOOD THAT THE FIRST NUMBER REFERS TO HORIZONTAL DISTANCE AND THE SECOND NUMBER REFERS TO VERTICAL DISTANCE (I.E., 2:1 IS 2 HORIZONTAL TO 1 VERTICAL).
- 14. GRADING SHALL BE PERFORMED IN SUCH A MANNER AS TO PREVENT WATER FROM FLOWING DIRECTLY OVER THE TOP OF ANY SLOPE. NO SLOPE SHALL BE LEFT TO STAND THROUGH A WINTER SEASON WITHOUT EROSION CONTROL MEASURES BEING PROVIDED.
- 15. PROVIDE AND INSTALL PERFORATED PIPES AND FILTER MATERIAL FOR SUBDRAINS, AS SHOWN ON THE GRADING PLANS OR AS DIRECTED BY THE ENGINEER.
- 16. PERFORATED SDR-35 PVC PIPE SHALL BE USED WITH THE PERFORATIONS PLACED DOWN. USE OF OTHER MATERIALS WILL BE PERMITTED ONLY UPON AUTHORIZATION OF THE ENGINEER.
- 17. THE ALLOWED PERMEABLE FILTER MATERIAL WILL BE CLASS 2 PERMEABLE MATERIAL AS SPECIFIED IN SECTION 68-1.025 OF THE STANDARD SPECIFICATIONS OF THE STATE OF CALIFORNIA OR $\frac{3}{4}$ " INCH CLEAN DRAIN ROCK WRAPPED IN FILTER FABRIC.
- 18. USE PIPES OF NO LESS THAN 4 INCHES IN DIAMETER. THE USE OF WYES, ELBOWS, TEES, CLEANOUTS, OR OTHER PIPE FITTINGS SHALL BE ALLOWED AT THE DISCRETION OF THE ENGINEER BASED ON FIELD CONDITIONS.
- 19. NON-PERFORATED PVC PIPE SHALL BE USED AT THE OUTLET OF ALL SUBDRAINS AT THE TOE OF THE ENGINEERED FILL SLOPES AND AT OTHER LOCATIONS WHEN REQUIRED BY THE ENGINEER. COMPACTED ENGINEERED TRENCH BACKFILL USING NATIVE SOILS MAY BE REQUIRED BY THE ENGINEER IN LIEU OF PERMEABLE MATERIAL IN LOCATIONS WHERE NON-PERFORATED PIPE IS SPECIFIED.
- 20. EROSION CONTROL MEASURES SHALL BE PROVIDED IN ACCORDANCE WITH THE COUNTY REQUIREMENTS.
- 21. THE LOCATION OF THE SUBDRAIN CLEANOUTS SHALL BE MARKED WITH PLASTIC CAPS AT THE GROUND SURFACE. THE CLEANOUTS SHALL BE IN LINE WITH THE SUBDRAIN AND DRAIN LINES SO THAT THE LOCATION OF BURIED DRAIN LINES CAN BE READILY DETERMINED

SPECIAL INSPECTION NOTES

- 1. INSPECTIONS OF THE TIEBACK DRILLING AND VERTICAL PIER HOLE DRILLING ARE REQUIRED TO BE PERFORMED BY THE REGISTERED SOIL ENGINEER OF RECORD.
- 2. SCOPE OF INSPECTION INCLUDE:
 - a. INSPECT INSTALLATION OF TIEBACK AND PIER HOLE DEPTH.
 - b. OBSERVE ALL PULLOUT (POST-TENSIONING) TESTING TO ASCERTAIN DESIGN SPECIFICATION IS ACHIEVED.
 - c. INSPECT INSTALLATION OF STEEL REINFORCEMENT.

SAFETY NOTES

- 1. THE CONTRACTOR SHALL NOTIFY THE UNDERGROUND SERVICE ALERT (USA) A MINIMUM OF 2 WORKING DAYS PRIOR TO ANY DIGGING. CALL 1-800-642-2444.
- 2. THE CONTRACTOR SHALL COMPLY WITH ALL STATE, COUNTY, AND COUNTY LAWS AND ORDINANCES, AND REGULATIONS OF THE DEPARTMENT OF INDUSTRIAL RELATIONS O.S.H.A., AND INDUSTRIAL ACCIDENT COMMISSION RELATING TO THE SAFETY AND CHARACTER OF WORK EQUIPMENT AND LABOR PERSONNEL.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ALL PUBLIC RIGHTS-OF-WAY AND OFF-SITE AREAS FROM ALL DIRT, MUD, DUST, AND DEBRIS AT ALL TIMES.
- 5. THE CONTRACTOR SHALL PREVENT THE FORMATION OF ANY AIRBORNE NUISANCE BY WATERING DOWN AND/OR TREATING THE SITE OF THE WORK IN SUCH A MANNER THAT WILL CONFINE DUST PARTICLES TO THE IMMEDIATE WORK AREA. IN ADDITION, THE NOISE LEVEL OF OPERATION SHALL BE KEPT TO A MINIMUM AS PER THE COUNTY.
- 6. WORK SHALL BE SUSPENDED, IF IN THE OPINION OF THE COUNTY, DUST CONTROL MEASURES ARE NOT ADEQUATE.
- 7. SANITARY FACILITIES SHALL BE MAINTAINED ON THE SITE.

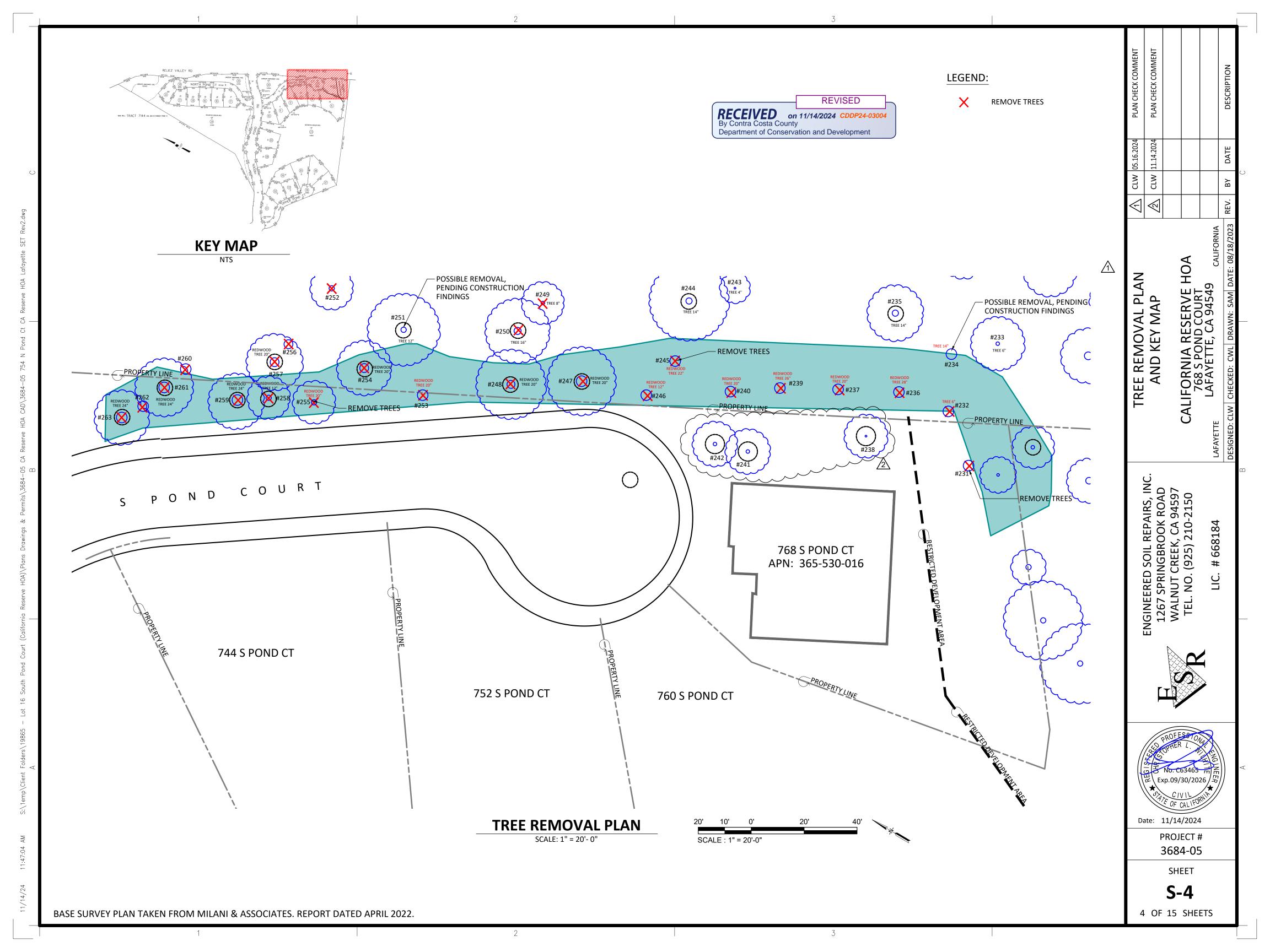


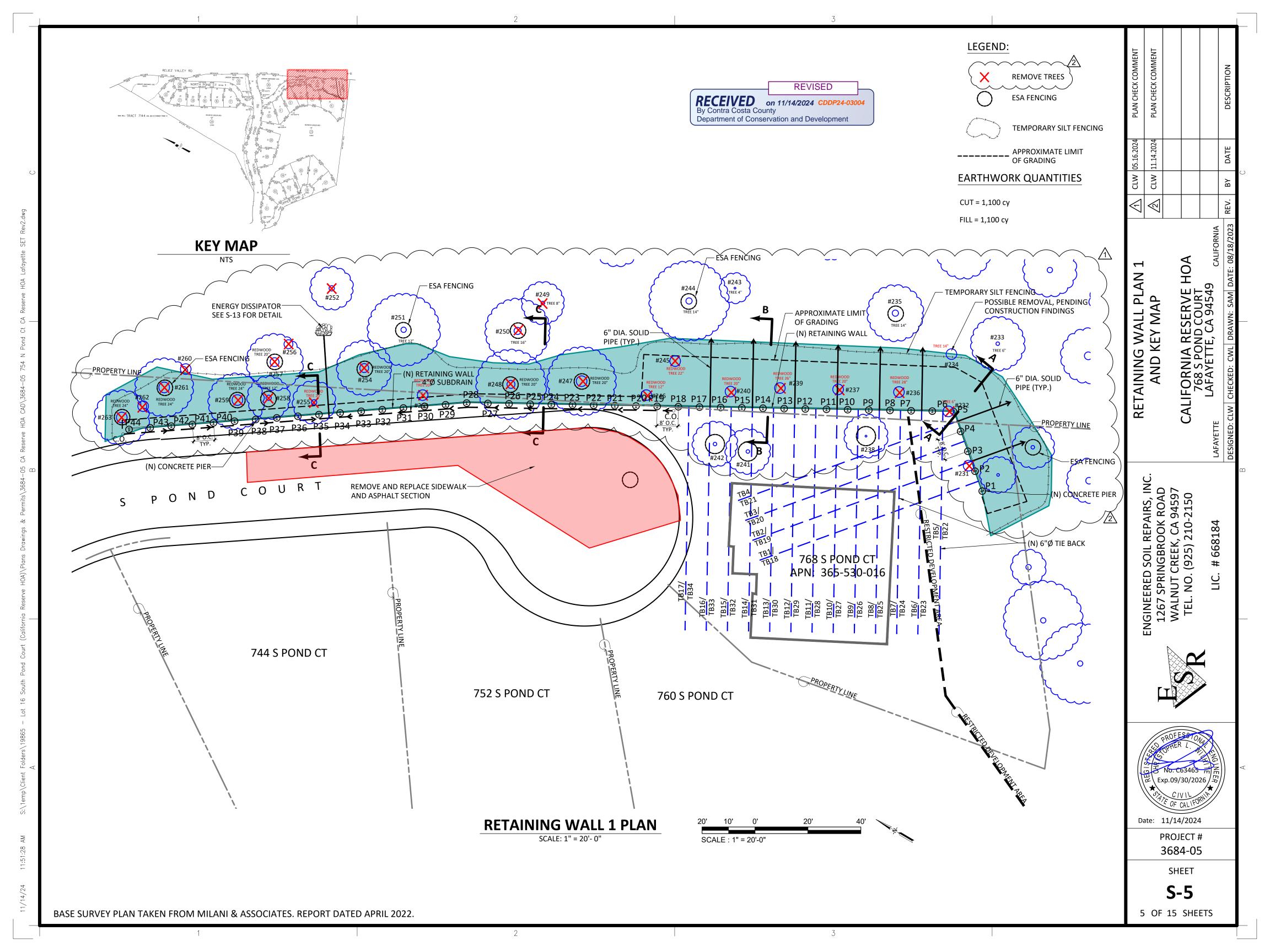


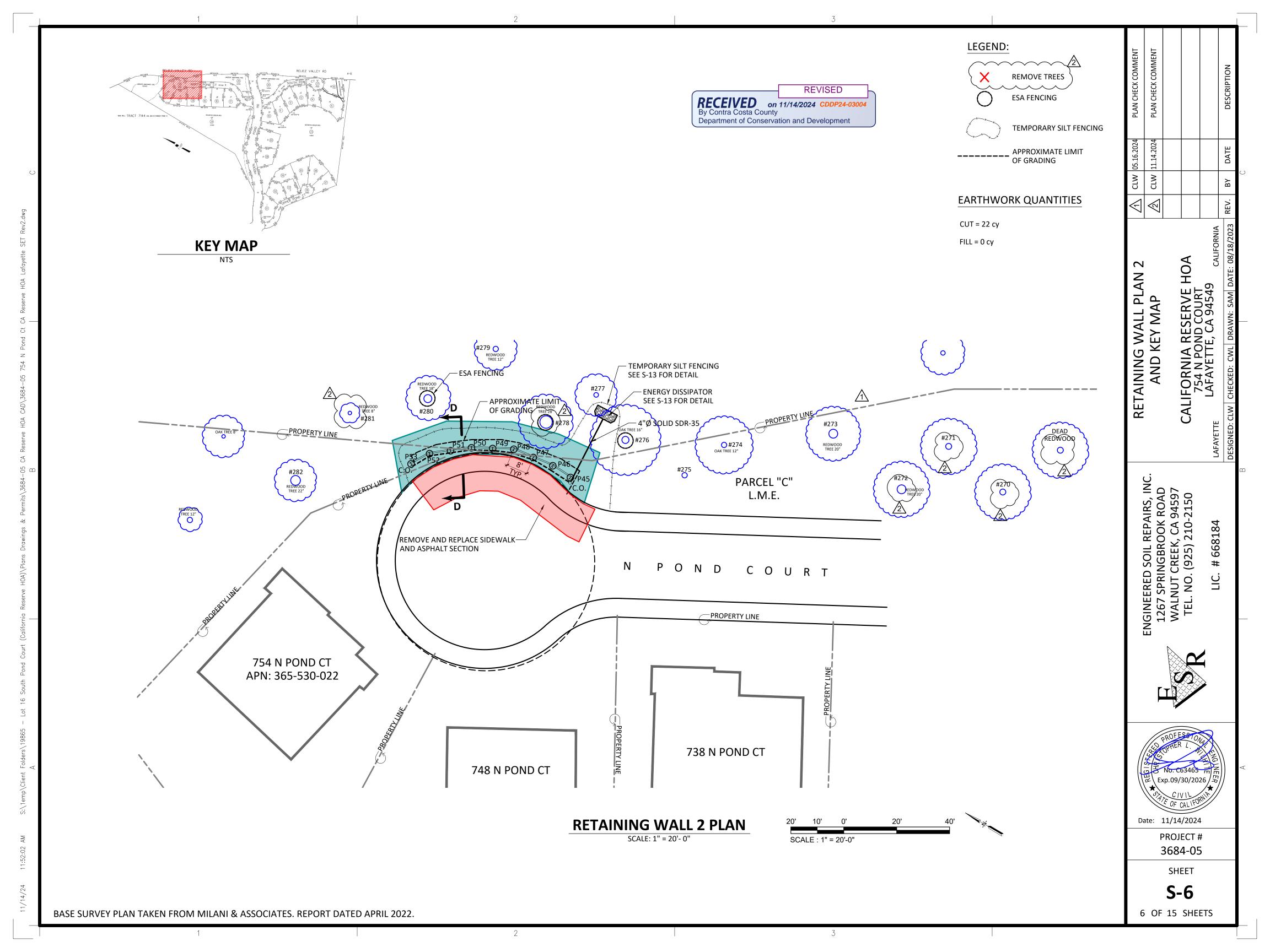
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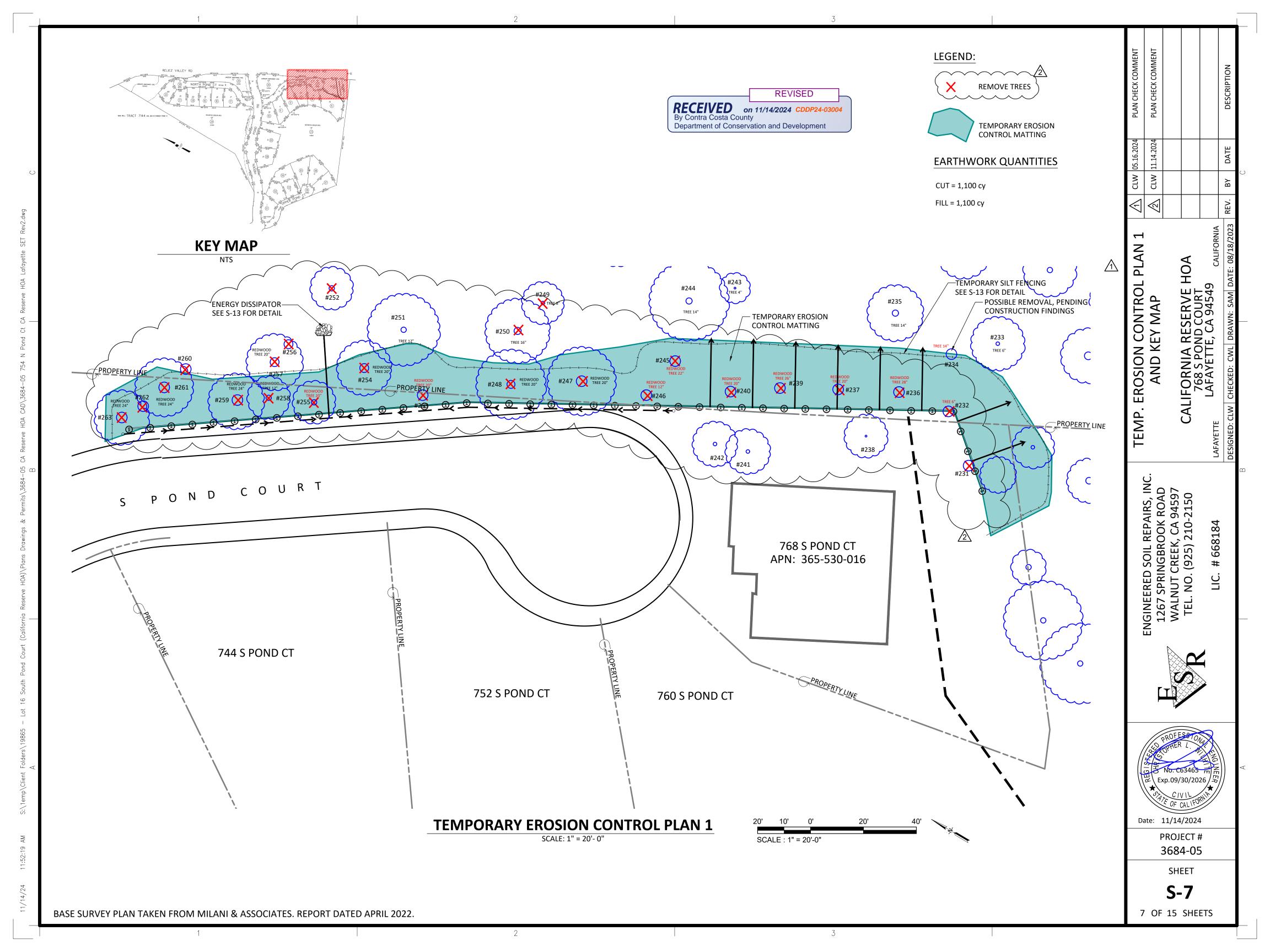
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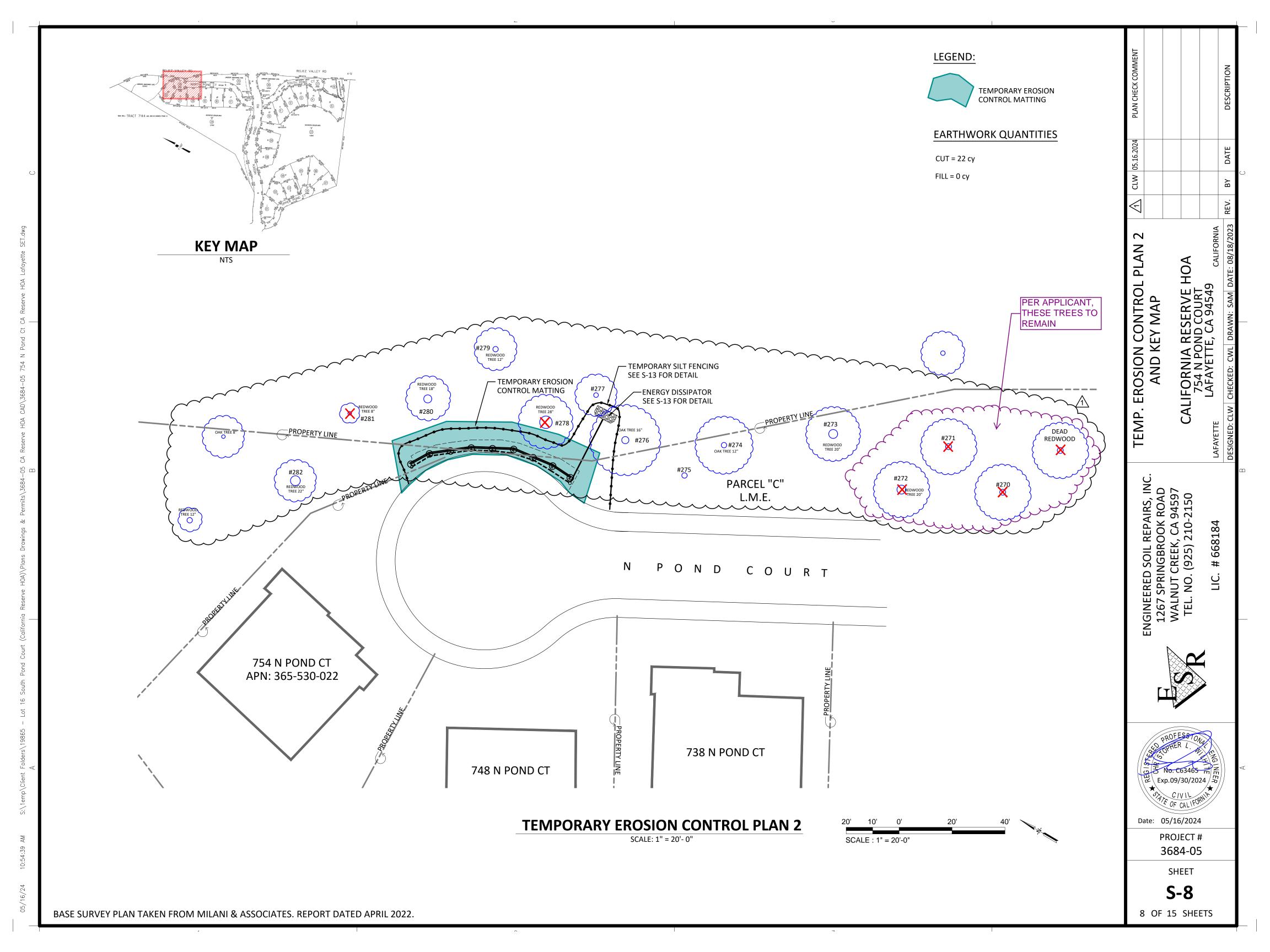
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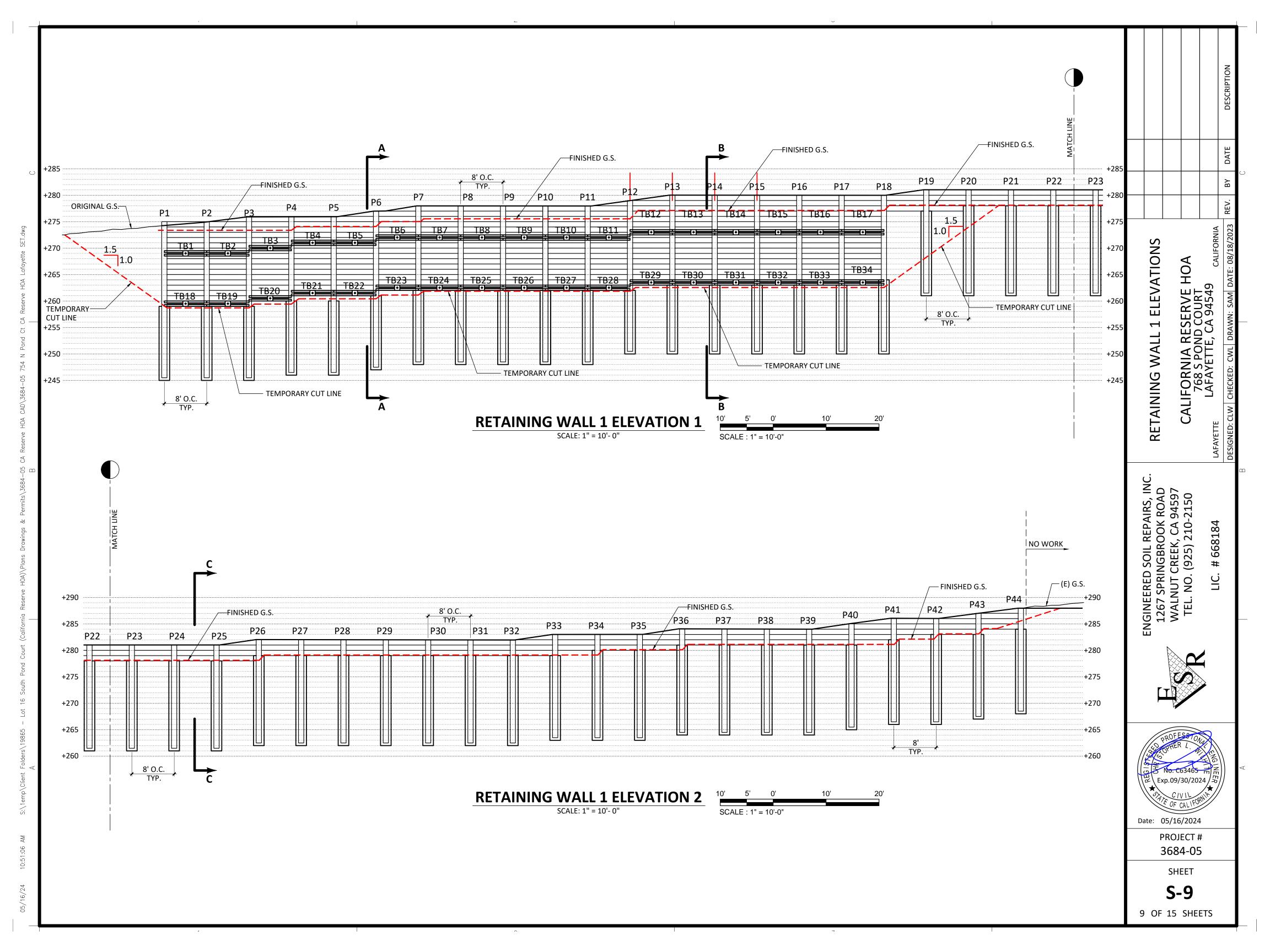


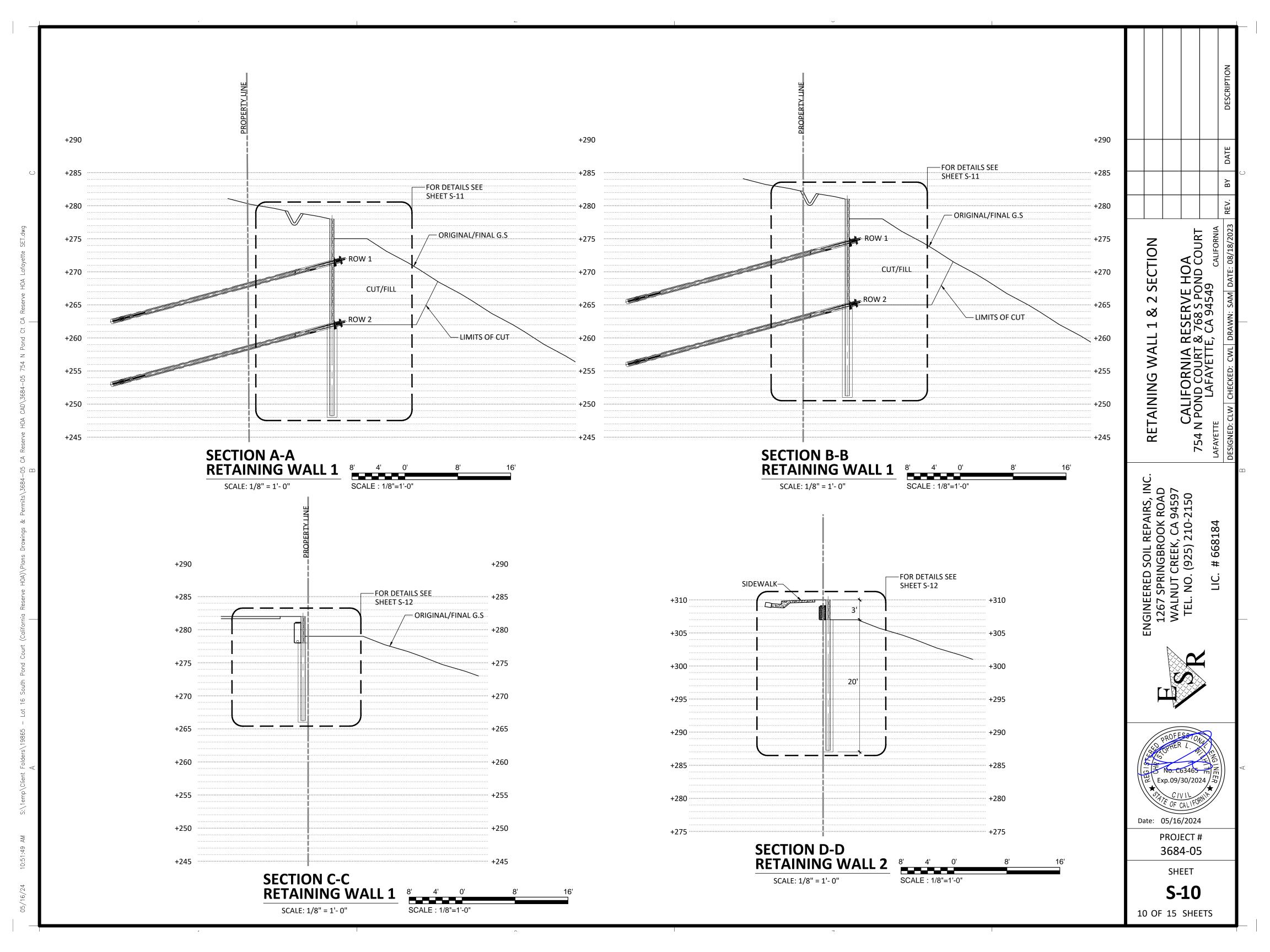


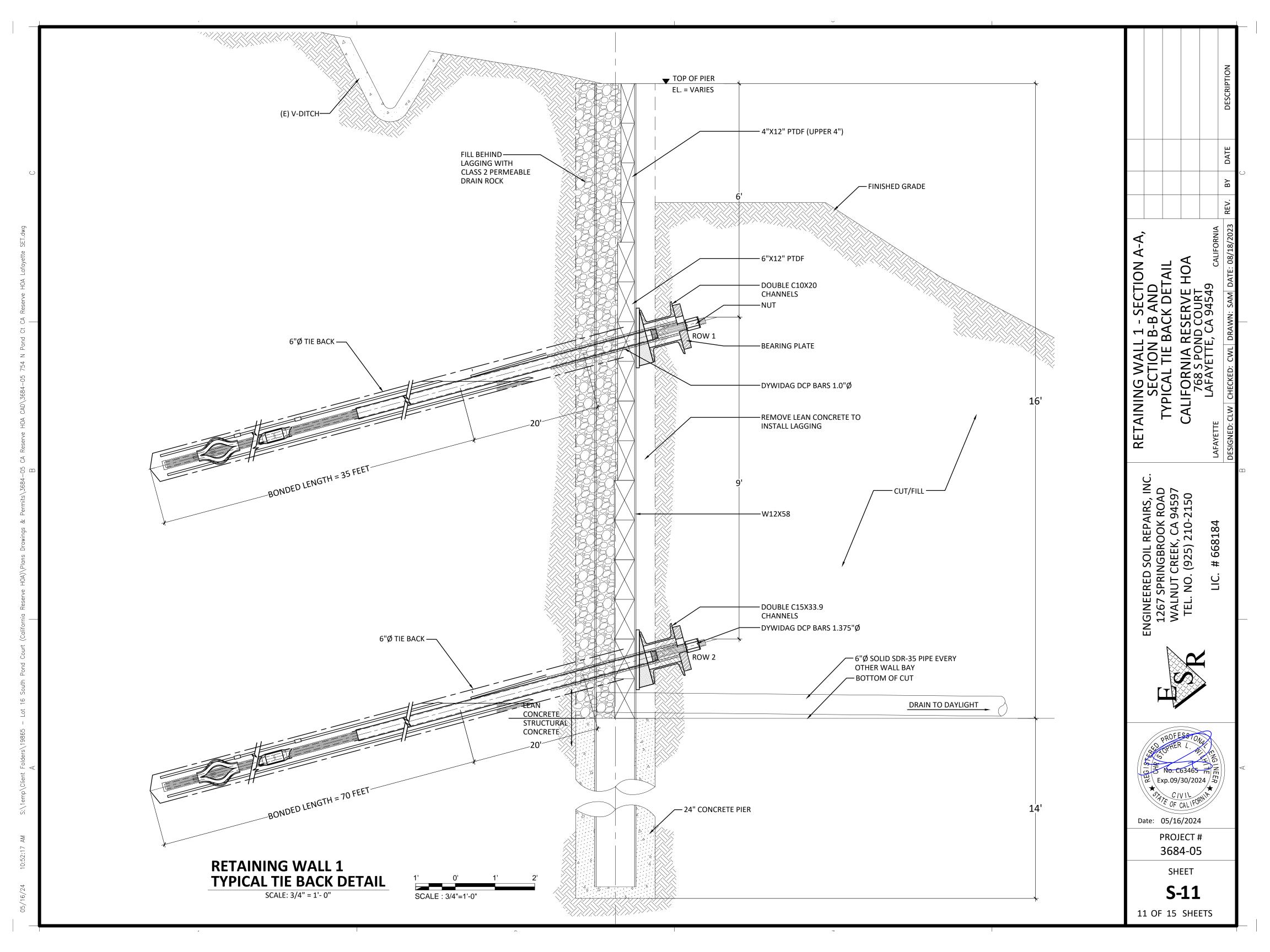


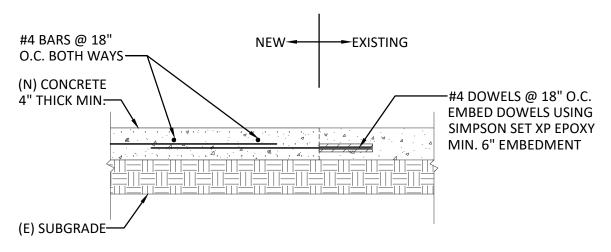






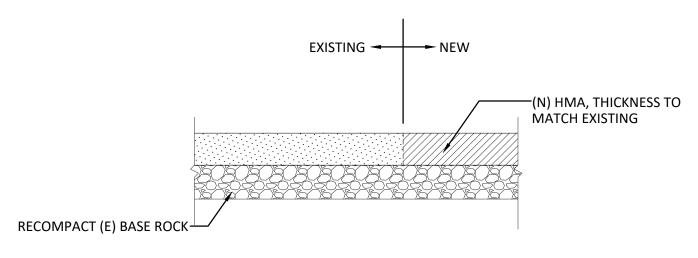






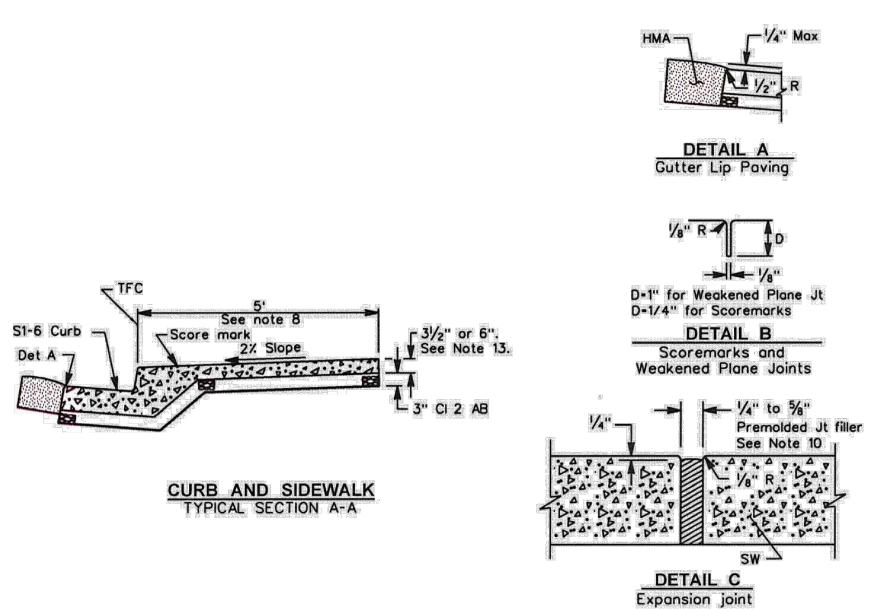
CONCRETE FLATWORK REPLACEMENT DETAIL

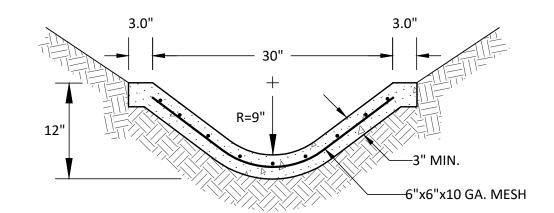
SCALE: 1"=1'-0"



ASPHALT REPLACEMENT DETAIL

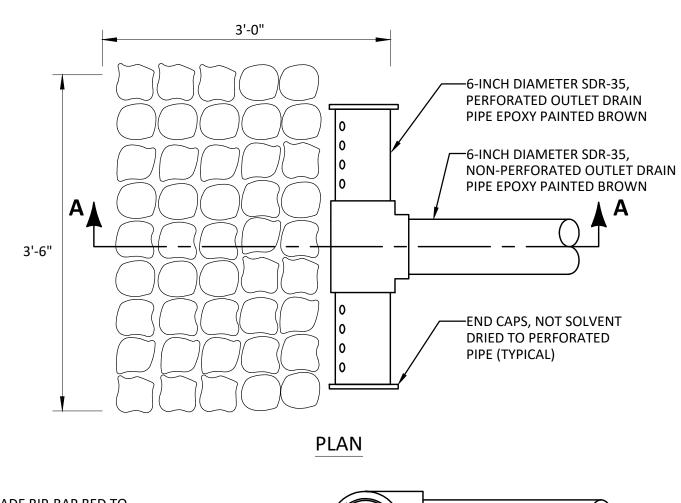
SCALE: 1"=1'-0"

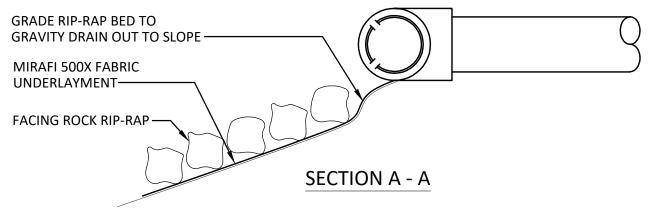




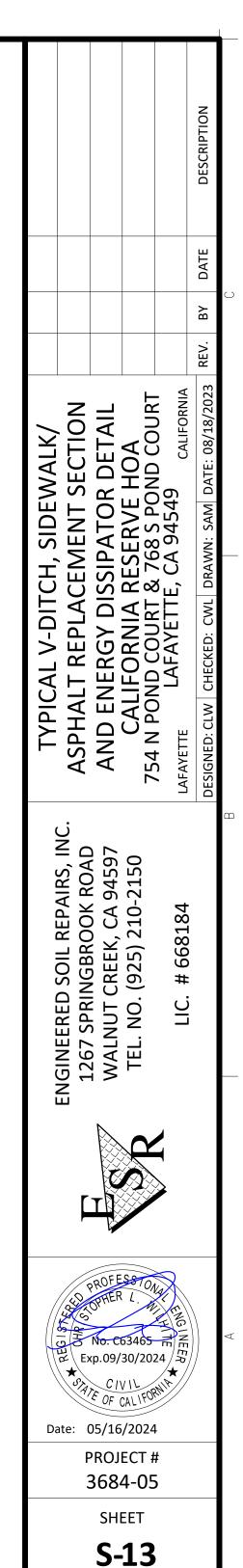
CONCRETE LINED DRAINAGE DITCH (AS REQUIRED)

SCALE : 1"=1'-0"

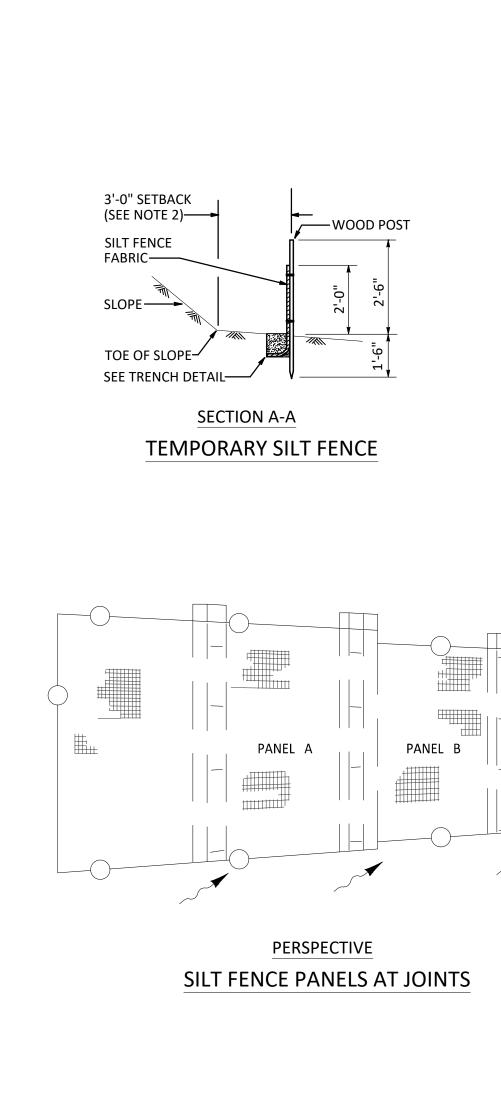


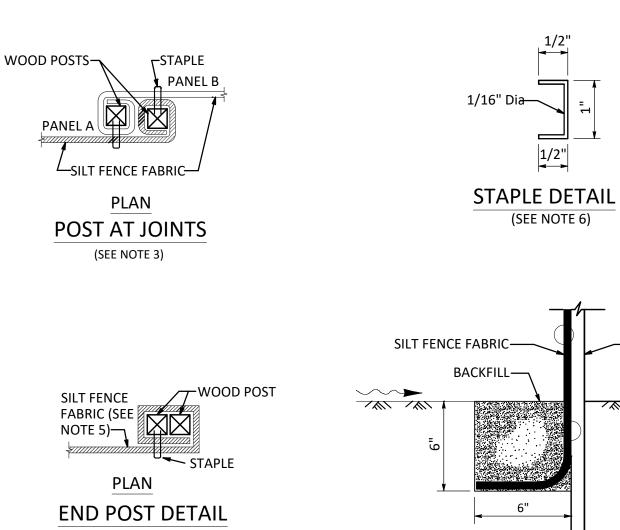


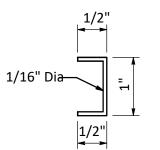
ENERGY DISSIPATER DETAIL

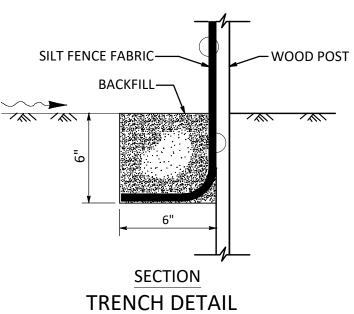






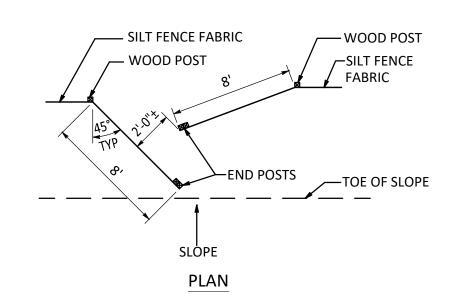




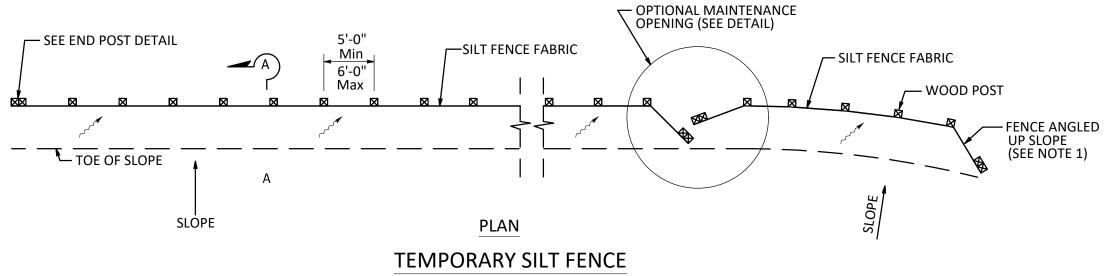


NOTES:

- 1. THE DOWN STREAM END OF THE TEMPORARY SILT FENCE SHALL HAVE THE LAST 8' ANGLED UP SLOPE.
- 2. SETBACK DIMENSIONS MAY VARY TO FIT FIELD CONDITIONS.
- 3. POSTS TO OVERLAP AND FENCE FABRIC TO FOLD AROUND EACH POST ONE FULL TURN. SECURE FABRIC WITH 4 STAPLES FOR EACH POST.
- 4. POSTS SHALL BE DRIVEN TIGHTLY TOGETHER TO PREVENT POTENTIAL FLOW-THROUGH OF SEDIMENT AT THE JOINT. THE TOPS OF THE POSTS SHALL BE SECURED TO EACH OTHER WITH WIRE.
- 5. FOR EACH END POST, FENCE FABRIC SHALL BE FOLDED AROUND TWO POSTS ONE FULL TURN AND SECURED WITH 4 STAPLES.
- 6. MINIMUM OF 4 STAPLES SHALL BE INSTALLED PER POST. DIMENSIONS SHOWN ARE TYPICAL.
- 7. MAINTENANCE OPENINGS SHALL BE CONSTRUCTED IN A MANNER TO ENSURE THAT SEDIMENT IS RETAINED BY THE TEMPORARY SILT FENCE.
- 8. JOINT SECTIONS SHALL NOT BE PLACED AT SUMP LOCATIONS.



OPTIONAL MAINTENANCE OPENING DETAIL



STAPLE SPACING

8" MAX

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

TEMPORARY WATER POLLUTION **CONTROL DETAILS** (TEMPORARY SILT FENCE)

NO SCALE

ERVE HOA 8 S POND COURT 94549

FENCE DETAIL SILT **TEMPORARY**

ENGINEERED SOIL REPAIRS, INC. 1267 SPRINGBROOK ROAD WALNUT CREEK, CA 94597 TEL. NO. (925) 210-2150

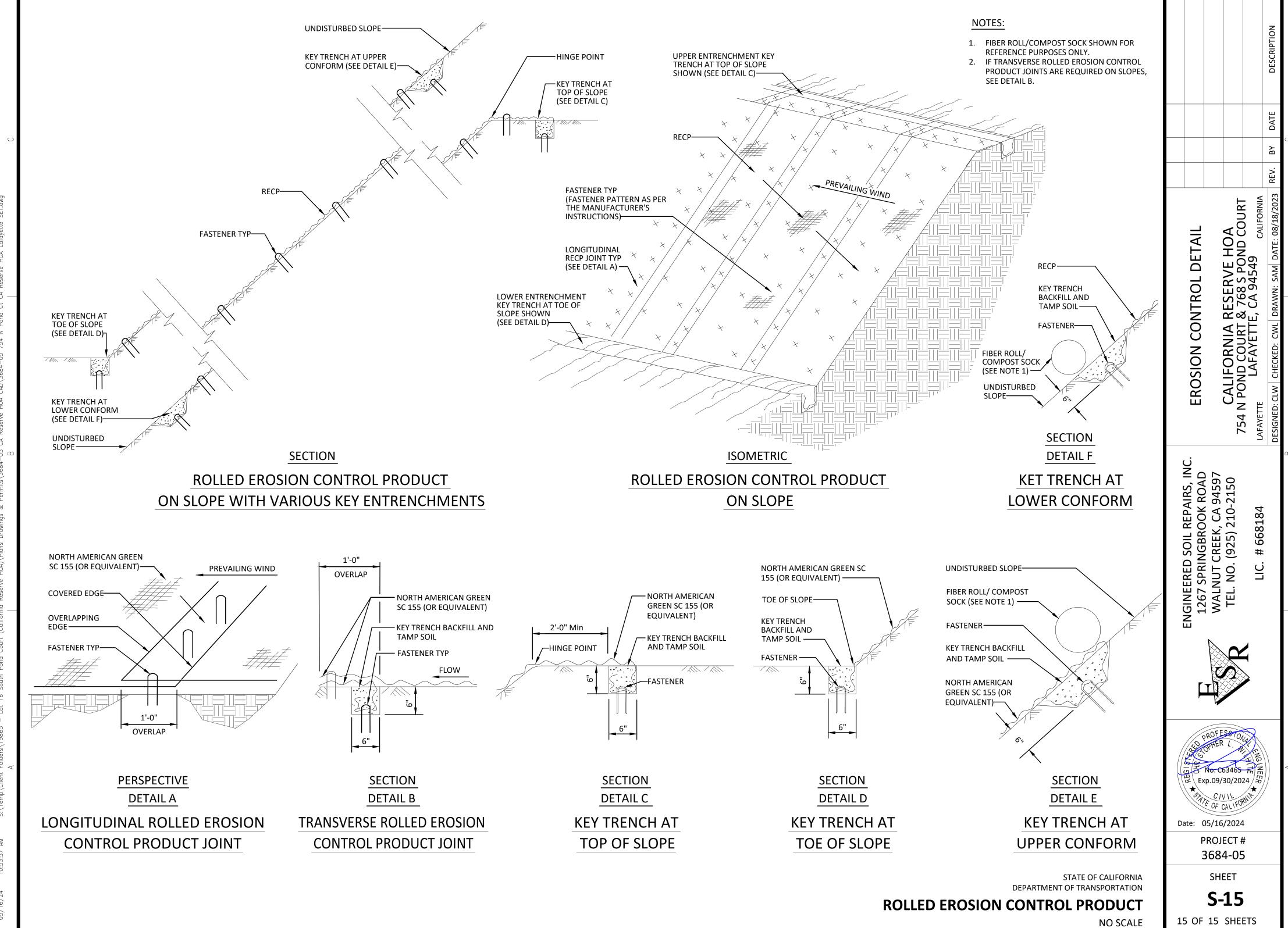


Date: 05/16/2024

PROJECT# 3684-05

SHEET

S-14



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